

ABBREVIATIONS:			
ABV.	-ABOVE	MANUF.	-MANUFACTURER
BOT.	-BOTTOM	MAS.	-MASONRY
B.W.	-BOTH WAYS	MAX.	-MAXIMUM
CONC.	-CONCRETE	MIN.	-MINIMUM
CONT.	-CONTINUOUS	NA	-NOT APPLICABLE
CANT.	-CANTILEVER	OC	-ON CENTER
CJ	-CEILING JOIST	REQ'D.	-REQUIRED
DBL	-CONCRETE MASONRY UNIT	RR	-ROOF RAFTER
DMU	-DOUBLE	SCH.	-SCHEDULE
DIA.	-DIAMETER	SIMP.	-SIMPSON
E.E.	-EACH END	THK.	-THICK
EMBED.	-EMBEDMENT	TRPL.	-TRIPLE
E.W.	-EACH WAY	TYP.	-TYPICAL
FDN	-FOUNDATION	UNO	-UNLESS NOTED OTHERWISE
FJ	-FLOOR JOIST	XJ	-EXTRA JOIST
FTG.	-FOOTING	W/	-WITH
HDG	-HOT DIPPED GALVANIZED	WWF	-WELDED WIRE FABRIC
HDR	-HEADER	VERT.	-VERTICAL
HORZ.	-HORIZONTAL	(#)	-NUMBER

GENERAL STRUCTURAL NOTES

- ALL CONSTRUCTION, WORKMANSHIP, AND MATERIALS SHALL CONFORM TO THE LATEST REQUIREMENTS OF "2018 INTERNATIONAL BUILDING CODE" AND LOCAL REGULATIONS.
- THE ENGINEER WHOSE SEAL APPEARS ON THESE DRAWINGS IS THE STRUCTURAL ENGINEER OF RECORD FOR THIS PROJECT. NO OTHER PARTY MAY MODIFY OR REUSE THESE CONSTRUCTION DOCUMENTS WITHOUT WRITTEN PERMISSION FROM WOODARD SEASE & ASSOCIATES, PC OR STRUCTURAL ENGINEER OF RECORD. ENGINEERS SEAL ONLY APPLIES TO STRUCTURAL COMPONENTS AND SYSTEMS AND DOES NOT CERTIFY DIMENSIONAL ACCURACY OF THE ARCHITECTURAL LAYOUT.
- THE ENGINEER SHALL HAVE NO LIABILITY TO OTHERS FOR ACTS OR OMISSIONS OF THE CONTRACTOR/BUILDER OR ANY OTHERS PERFORMING WORK ON THIS PROJECT. THE ENGINEER IS NOT RESPONSIBLE FOR CONSTRUCTION SEQUENCES, METHODS, OR TECHNIQUES AND/OR SAFETY REQUIREMENTS IN CONNECTION WITH THE CONSTRUCTION OF THIS STRUCTURE.
- THE CONTRACTOR ASSUMES ALL RESPONSIBILITY FROM DEPICTED OR IMPLIED STRUCTURAL INFORMATION. SHOULD ANY DISCREPANCIES BECOME APPARENT, THE STRUCTURAL ENGINEER OF RECORD MUST BE NOTIFIED IMMEDIATELY BEFORE CONSTRUCTION BEGINS.
- ONLY SEALED DRAWINGS W/LATEST REVISIONS ARE APPLICABLE FOR CONSTRUCTION.
- DEFLECTION: FLOOR: L/360, ATTIC W/ CEILING: L/240, ROOF: L/180 - MORE STRINGENT CRITERIA MAY BE USED AT ENGINEER'S DISCRETION OR AS REQUESTED.
- DO NOT SCALE DRAWINGS. CONTRACTOR SHALL CONTACT ARCHITECT FOR ITEMS NOT DIMENSIONED.
- THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL MEMBERS AS REQUIRED FOR THE STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT / ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS WITHIN THE STRUCTURE.
- CONSTRUCTION MATERIALS SHALL NOT BE STACKED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED IN THE DESIGN LOADS. IT IS THE GENERAL CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT THE SUBCONTRACTORS ARE INFORMED AND DO NOT VIOLATE THIS IMPORTANT REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOOR AND ROOFS.
- SEE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR MISCELLANEOUS STEEL ITEMS NOT SHOWN.
- COORDINATE SIZES AND LOCATIONS OF OPENINGS IN FLOORS AND ROOF WITH ARCHITECTURAL, MECHANICAL, AND ELECTRICAL REQUIREMENTS.
- FOR ACTUAL ELEVATION OF FIRST FLOOR (REF. ELEV. 0'-0"), SEE SITE PLAN.
- SUBMIT WRITTEN REQUEST TO THE ARCHITECT FOR APPROVAL OF ANY PROPOSED CHANGE TO THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. SPLICING, CUTTING, NOTCHING OR OTHER ALTERATIONS TO THE STRUCTURAL MEMBERS ARE NOT PERMITTED WITHOUT WRITTEN AUTHORIZATION OF THE ENGINEER. ANY UNAUTHORIZED DEVIATION FROM THE CONTRACT DOCUMENTS, AND CORRECTION THEREOF, IS THE RESPONSIBILITY OF THE CONTRACTOR.
- THE MOST STRINGENT REQUIREMENTS APPLY IN CASE OF CONFLICT BETWEEN SPECIFICATIONS, STANDARDS, CODES, AND DRAWINGS.

DESIGN LOADS - BUILDINGS

FLOOR/ROOF AREAS	LIVE LOAD	DEAD LOAD	
ROOF	20 PSF	10 PSF	(TOP CHORD)
	10 PSF	10 PSF	(BOTTOM CHORD)
FLOOR (SLAB ON GRADE)	250 PSF		

OCCUPANCY CATEGORY : II
WIND DESIGN:

BASIC WIND VELOCITY =	115 MPH
IMPORTANCE FACTOR = Iw =	1.00
EXPOSURE CATEGORY =	C
BUILDING CATEGORY =	ENCLOSED
BUILDING 100	
WIND BASE SHEAR Vx =	8.3 KIPS
WIND BASE SHEAR Vy =	8.3 KIPS
BUILDING 200	
WIND BASE SHEAR Vx =	8.3 KIPS
WIND BASE SHEAR Vy =	12.8 KIPS

SNOW LOADS:

GROUND SNOW LOAD Pg =	10 PSF
EXPOSURE FACTOR Ce =	1.0
THERMAL FACTOR Ct =	1.2
IMPORTANCE FACTOR = Ipg =	1.0
SNOW LOAD Pf =	8.4 PSF

SEISMIC LOADS:

IMPORTANCE FACTOR = Ie =	1.00
SEISMIC DESIGN CATEGORY =	B
OCCUPANCY =	II
SITE CLASS =	D
BASIC STRUCTURAL SYSTEM =	MASONRY BEARING WALL
RESPONSE MOD = R =	6.5
Ss =	35 %g
S1 =	13 %g
ANALYSIS PROCEDURE =	EQUIVALENT
BUILDING 100	
DESIGN BASE SHEAR =	Vx = 11.9 KIPS, Vy = 11.9 KIPS
BUILDING 200 & 300	
DESIGN BASE SHEAR =	VX = 11.6 KIPS, VY = 11.6 KIPS

MASONRY:

- MASONRY NOTES
- CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C 90, GRADE N-1 UNLESS OTHERWISE NOTED, COMPRESSIVE STRENGTH ON NET CROSS SECTIONAL AREA: 2000 PSI.
 - MASONRY SHALL BE LAID IN ASTM C 270, TYPE "S" MORTAR, UNLESS OTHERWISE NOTED AND SHALL HAVE FULL MORTAR COVERAGE OF THE FACE SHELLS IN BOTH HORIZONTAL AND VERTICAL JOINTS.
 - GROUT FOR REINFORCED MASONRY SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS AND SHALL CONFORM TO ASTM C476.
 - GROUT FOR REINFORCED MASONRY SHALL HAVE A SLUMP OF 8 TO 11 INCHES. COURSE GROUT SHALL BE USED IN SPACES 2-1/2" X 3" AND GREATER. FINE GROUT SHALL BE USED IN SPACES 2" X 3" AND SMALLER.
 - MAXIMUM HEIGHT TO WHICH MASONRY SHALL BE LAID BEFORE FILLING IS 4 FEET FOR COARSE GROUT AND 2 FEET FOR FINE GROUT.
 - REINFORCEMENT GRADE AND DETAILS SHALL BE THE SAME AS FOR CONCRETE. TIE IN POSITION AND PLACE CONCRETE AROUND REINFORCING DURING CONSTRUCTION OF MASONRY. DO NOT PUSH REINFORCING DOWN INTO PREVIOUSLY PLACED GROUT FILL. SET BOLTS SIMILARLY.
 - HORIZONTAL REINFORCING BARS MAY BE SPLICED WITH A MINIMUM LAP OF 48 TIMES THE BAR DIAMETER UNO
 - REINFORCE ALL WALLS WITH 9 GAGE CONTINUOUS LADDER TYPE REINFORCING AT 16" VERTICAL SPACING UNO ON PLANS.
 - SPLICED REINFORCING BARS SHALL OCCUPY THE SAME CELL. MINIMUM LAP SPLICE SHALL BE 48 TIMES THE BAR DIAMETER. SPLICED BARS NEED NOT BE TIED TOGETHER.
 - VERTICAL REINFORCEMENT IN WALLS SHALL BE SUPPORTED AND SECURED AGAINST DISPLACEMENT AT 6 FOOT INTERVALS FOR #3 AND #4 BARS AND 8 FOOT INTERVALS FOR #5 AND #6 BARS.
 - WHERE INTERIOR CONCRETE MASONRY PARTITIONS INTERSECT WITH OTHER INTERIOR PARTITIONS OR EXTERIOR WALLS, A MASONRY BOND, OR THE EQUIVALENT IN APPROVED METAL TIES, SHALL BE PROVIDED UNO ON THE DRAWINGS.
 - MORTAR JOINTS SHALL BE 3/8" THICK WITH FULL MORTAR COVERAGE ON VERTICAL AND HORIZONTAL FACE SHELLS. VERTICAL JOINTS SHALL BE SHOVED TIGHT.
 - BACKFILLING AGAINST MASONRY WALLS SHALL NOT BE PERMITTED UNTIL SUFFICIENT LATERAL SUPPORT IS PROVIDED BY THE CONTRACTOR.
 - PROVIDE BOND BEAMS AT MASONRY ELEVATIONS AS SHOWN ON THE ARCHITECTURAL AND STRUCTURAL DRAWINGS. BOND BEAMS SHALL HAVE TWO #4 BARS, CONTINUOUS, UNLESS OTHERWISE NOTED.
 - ALL VERTICAL REINFORCING BARS SHALL EXTEND 6" MINIMUM INTO BOND BEAM AT TOP OF WALL.

STRUCTURAL STEEL:

- STEEL NOTES
- ALL STEEL IS TO BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION'S "MANUAL OF STEEL CONSTRUCTION" AND "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."
 - PRODUCTS:
STRUCTURAL STEEL: ASTM A992/ASTM 572 Fy = 50 ksi
TUBULAR STEEL: ASTM A500 B Fy = 46 ksi
CONNECTION BOLTS: ASTM 325 (THREADS INCLUDED)
ANCHOR BOLTS: ASTM A307
WELD ELECTRODES: E70XX OR AWS EQUIVALENT
SHOP PRIMER: ONE COAT OF RUST INHIBITIVE PRIMER
 - USE A-325 BOLTS FOR ALL STEEL CONNECTIONS UNLESS OTHERWISE NOTED. ALL CONNECTIONS ARE BEARING TYPE AND BOLT THREADS ARE INCLUDED FROM SHEAR PLANE UNLESS OTHERWISE SPECIFIED.
 - USE A-307 ANCHOR BOLTS UNLESS OTHERWISE NOTED. ALL ANCHOR BOLTS SHALL HAVE STANDARD HOOK OF NOT LESS THAN 3" UNLESS SPECIFIED OTHERWISE.
 - BOLT HOLE NOTATION AND SPECIFICATION:
d = BOLT DIAMETER
STD = STANDARD HOLE = d + 1/16"
OVS = OVERSIZED HOLE = d + 5/16"
SSL = SHORT SLOTTED HOLE = (d + 1/16") X (d + 3/8")
LSL = LONG SLOTTED HOLE = (d + 1/16") X (d + 2.5Xd)
NSL = LONG OR SHORT SLOTTED HOLE NORMAL TO LOAD DIRECTION
 - CONNECTION TYPE NOTATION AND SPECIFICATION:
SC - SLIP CRITICAL
N - BEARING TYPE W/ THREADS INCLUDED IN SHEAR PLANE
X - BEARING TYPE W/ THREADS EXCLUDED FROM SHEAR PLANE
 - ALL WELDING SHALL CONFORM TO THE LATEST EDITION OF THE AMERICAN WELDING SOCIETY'S "STRUCTURAL WELDING CODE FOR STEEL." ALL WELDERS SHALL BE CERTIFIED PER AWS.

SPECIAL INSPECTIONS:

- SPECIAL INSPECTIONS
- OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS DURING CONSTRUCTION ON THE TYPES OF WORK LISTED PER SECTION 1704.1.12. THESE ARE IN ADDITION TO INSPECTIONS BY NC ADMINISTRATIVE CODE AND POLICIES.
 - SPECIAL INSPECTOR SHALL BE A PERSON WHO SHALL DEMONSTRATE COMPETENCY TO THE SATISFACTION OF THE BUILDING CODE OFFICIAL.
 - SPECIAL INSPECTIONS PER CH. 17 OF IBC 2012 ARE REQ'D WHEN ANY OF THE FOLLOWING ARE MET
 - BUILDING OCCUPANCY II AND BUILDING HEIGHT EXCEEDS 45 FT OR 3 STORIES OR BUILDING IS AN UNDERGROUND BUILDING PER SECTION 405.1.
 - BUILDING OR OTHER STRUCTURE IS OCCUPANCY CATEGORY III OR IV.
 - PILES, PIERS AND SPECIAL FOUNDATIONS IN ACCORDANCE WITH 1704.8 THRU 1704.11, 1810.3.5.2.4 AND 1810.3.5.2.5.
 - RETAINING WALLS EXCEEDING 5 FEET IN HEIGHT PER SECTION 1807.2.
 - SMOKE CONTROL AND SMOKE EXHAUST SYSTEMS.
 - SPRAYED FIRE-RESISTANT MATERIAL.
 - SPECIAL CASE DESCRIBED IN SECTION 1704.15.
 - SPECIAL INSPECTORS SHALL KEEP RECORDS OF INSPECTIONS AND PROVIDE TO BUILDING OFFICIAL AND REGISTERED DESIGN PROFESSIONAL PER SECTION 1704.1.3.
 - STEEL CONSTRUCTION IF REQUIRED SPECIAL INSPECTIONS PER 1704.1.2 SHALL BE PROVIDED PER SECTION 1704.3 AND TABLE 1704.3 ON A CONTINUOUS AND PERIODIC BASIS AS REQUIRED.
 - CONCRETE CONSTRUCTION IF REQUIRED SPECIAL INSPECTIONS PER 1704.1.2 SHALL BE PROVIDED PER SECTION 1704.4 AND TABLE 1704.4 ON A CONTINUOUS AND PERIODIC BASIS AS REQUIRED.
 - MASONRY CONSTRUCTION IF REQUIRED SPECIAL INSPECTIONS PER 1704.1.2 SHALL BE PROVIDED PER SECTION 1704.5 AND TABLE 1704.5.1, 1704.5.3 ON A CONTINUOUS AND PERIODIC BASIS AS REQUIRED.
 - WOOD CONSTRUCTION IF REQ'D SPECIAL INSPECTIONS PER 1074.1.2 SHALL BE PROVIDED PER SECT. 1704.6.
 - WOOD CONSTRUCTION IF REQ'D SPECIAL INSPECTIONS PER 1074.1.2 SHALL BE PROVIDED PER SECT. 1704.6.
 - SOIL SPECIAL INSPECTIONS SPECIAL INSPECTIONS SHALL BE PROVIDED PER SECT. 1704.7 & TABLE 1704.7.
 - DRIVEN DEEP FOUNDATIONS SPECIAL INSPECTIONS SHALL BE PROVIDED PER SECT. 1704.8 & TABLE 1704.8.
 - CAST IN PLACE DEEP FOUNDATIONS SPECIAL INSPECTIONS SHALL BE PROVIDED PER SECTION 1704.9 AND TABLE 1704.9.
 - HELICAL PILE FOUNDATIONS SPECIAL INSPECTIONS SHALL BE PROVIDED PER SECTION 1704.10.
 - SPECIAL INSPECTIONS FOR WIND REQUIREMENTS IF REQUIRED PER SECTION 1704.1.2 AND SECTION 1706.2 THRU 1706.4 PER SECTION 1706 WHEN:
 - EXPOSURE B AND WIND SPEED IS 120 MPH OR GREATER
 - EXPOSURE C OR D AND WIND SPEED IS 110 MPH OR GREATER
 - SPECIAL INSPECTIONS FOR SEISMIC RESISTANCE IF REQUIRED PER SECTION 1704.1.2 AND SECTION 1707.2 THRU 1707.9 PER SECTION 1707 WHEN:
 - SEISMIC DESIGN CATEGORY C, D, E OR F AS DETERMINED IN SECTION 1613.
 - DESIGNATED SEISMIC SYSTEMS IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY D,E OR F.
 - ARCHITECTURAL, MECHANICAL AND ELECTRICAL COMPONENTS IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E OR F THAT ARE REQUIRED IN SECTIONS 1707.6 AND 1707.7.

FABRICATION AND ERECTION NOTES

- FABRICATION AND ERECTION NOTES
- SHOP FABRICATION AND ERECTION IN ACCORDANCE WITH AISC CODE OF STANDARD PRACTICE.
 - QUALITY CONTROL SHALL BE PER ASIC "QUALITY CRITERIA AND INSPECTION STANDARDS".
 - ALL HOLES TO BE DRILLED HOLES TO BE DRILLED OR PUNCHED. DO NOT PRODUCE HOLES OR ENLARGE HOLES BY MEANS OF BURNING OR FLAME TORCHING.
 - SET STRUCTURAL STEEL ACCURATELY TO LINES AND ELEVATIONS AS SPECIFIED AND ADJUST PRIOR TO FINAL FASTENING WHETHER BOLTING OR WELDING.
 - SPLICES SHALL BE ALLOWED ONLY WHERE DENOTED UNLESS APPROVED.
 - TEMPORARY SHORING AND BRACING SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR TO SAFELY RESIST ALL CONSTRUCTION AND SHORT TERM LOADS PRIOR TO COMPLETE INSTALLATION OF BUILDING COMPONENTS.
 - GROUT BELOW BASE PLATES USING NON-SHRINK GROUT.

SLAB NOTES:

- SLAB NOTES
- MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE: 3000 PSI (NORMAL WEIGHT CONCRETE, I.E. 145 PCF)
 - ALL DETAILING, FABRICATION AND ERECTION OF REINFORCING BARS SHALL CONFORM TO ACT "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURE" (ACI-315) AND THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI-318).
 - REINFORCING BARS SHALL BE ROLLED FROM NEW BILLET STEEL CONFORMING TO "SPECIFICATION FOR DEFORMED BILLET STEEL BARS FOR CONCRETE REINFORCEMENT", ASTM A 615, AND SHALL BE GRADE 60 EXCEPT FOR COLUMN AND PILASTER TIES AND BEAM STIRRUPS WHICH SHALL BE GRADE 40.
 - WELDED WIRE FABRIC SHALL CONFORM TO ASTM A 185 AND A 82.
 - ALL SHOP AND FIELD WELDING OF REINFORCING STEEL TO STRUCTURAL SHAPES SHALL BE PERFORMED BY WELDERS WHO HAVE BEEN QUALIFIED BY TEST AS PRESCRIBED IN THE AWS D1.1-88 AND SHALL BE PERFORMED IN ACCORDANCE WITH AWS D1.4-79.
 - GROUT UNDER ALL COLUMN BASE PLATES AND BEAM BEARING PLATES WITH NON-SHRINK, NON-METALLIC GROUT WHICH CONFORMS TO ASTM C1107.
 - DETAILS, WORKMANSHIP AND PROCEDURE OF CONCRETE PLACEMENT SHALL CONFORM TO THE LATEST EDITIONS OF ACI-315, ACI-318 AND ACI-301.
 - CLEAR DISTANCE FROM FACE OF CONCRETE TO MAIN REINFORCING:
SUSPENDED SLABS AND JOISTS: 1" UNO
GRADE BEAMS, PEDESTALS, COLUMNS, WALLS: 2" UNO
FOOTINGS, WALLS CAST AGAINST EARTH: 3" UNO
 - ALL ISOLATION JOINT STRIPS SHALL BE 1/2" THICK, UNLESS OTHERWISE NOTED.
 - PROVIDE CORNER BARS AT ALL FOOTING STEPS AND CORNERS UNLESS OTHERWISE NOTED. BARS SHALL LAP A MINIMUM OF 48 BAR DIAMETERS AND SHALL HAVE THE SAME SIZE AND SPACING AS THE HORIZONTAL REINFORCING.
 - PROVIDE #4 DIAGONAL CORNER BARS, MINIMUM 48" LONG AT CENTER OF SLAB AROUND ALL OPENINGS IN FLOOR SLABS.
 - LAP ALL REINFORCING SPLICES AT LEAST 48 BAR DIAMETERS (24" MINIMUM) UNLESS OTHERWISE NOTED.
 - WELDED WIRE FABRIC SHALL HAVE END LAPS OF ONE FULL FABRIC PLUS 2" BETWEEN CROSS WIRES AND EDGE LAPS OBTAINED BY OVERLAPPING LONGITUDINAL SELVAGE WIRES 2" AND WIRING ALL LAPS SECURELY TOGETHER. WELDED WIRE FABRIC SHALL EXTEND INTO SUPPORTING BEAMS AND WALLS FOR ANCHORAGE UNLESS AN EXPANSION JOINT IS CALLED FOR.
 - ALL REINFORCING SHALL BE SECURELY WIRE TOGETHER IN FORMS AS CALLED FOR IN "PLACING REINFORCING BARS" BY CRSI.
 - CRACK CONTROL JOINTS SHALL BE PLACED IN SLABS ON GRADE AT A MAXIMUM SPACING OF 25', UNLESS OTHERWISE NOTED. 1/4" WIDTH AD 1/4 OF DEPTH TYP.
 - REINFORCING STEEL IN PLACE SHALL BE REVIEWED BY THE ARCHITECT AND/OR ENGINEER PRIOR TO PLACING CONCRETE.
 - CHAMFER EXPOSED EDGES OF CONCRETE 3/4" OR AS SHOWN IN ARCHITECTURAL DRAWINGS.
 - SLAB ON GRADE SHALL BE 4" THICK WITH W1.4 X W1.4 WELDED WIRE FABRIC AT 1/3 DEPTH FROM TOP OF SLAB UNLESS OTHERWISE NOTED.
 - PROVIDE ALL NECESSARY COVER AND PROTECTION FOR MASONRY WORK WHEN PLACING CONCRETE.
 - PROVIDE AT LEAST TWO (2) #4 BARS IN TOP OF WALL FOOTING UNDER DOOR AND OTHER OPENINGS, 4'-0" LONGER THAN THE OPENING.
 - PROVIDE DOWELS IN WALL FOOTINGS EQUIVALENT IN SIZE AND NUMBER TO VERTICAL STEEL EXTENDING 24 BAR DIAMETERS INTO FOOTING AND WITH PROJECTION AS REQUIRED TO PROVIDE MINIMUM LAP SPLICE AS NOTED ABOVE.
 - SEE ARCHITECTURAL FINISH SCHEDULE FOR REQUIRED FLOOR FINISHES.

WOOD TRUSS NOTES:

- WOOD TRUSS NOTES
- SHOP DRAWINGS FOR WOOD TRUSSES SHALL BEAR THE SEAL AND SIGNATURE OF AN ENGINEER LICENSED IN THE STATE OF NORTH CAROLINA AND SHALL BE APPROVED BY THE ENGINEER OR ARCHITECT BEFORE FABRICATION. SHOP DRAWINGS SHALL SHOW SPECIES AND MOISTURE CONTENT OF WOOD BEING USED. ALLOWABLE STRESSES SHALL CONFORM TO THE LATEST EDITION OF "NATIONAL DESIGN SPECIFICATION FOR STRESS - GRADE LUMBER AND ITS FASTENING", AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.
 - LUMBER IN TRUSSES SHALL BE GRADE MARKED, NO. 2 DIMENSION, KILN-DRIED SOUTHERN PINE, OR BETTER, UNLESS OTHERWISE INDICATED ON THE DETAILS EXCEPT TRUSS WEB MEMBERS MAY BE NO. 3, KILN-DRIED, GRADE MARKED, SOUTHERN PINE, UNLESS OTHERWISE INDICATED ON THE DRAWINGS. MAXIMUM ALLOWABLE MOISTURE CONTENT = 15%.
 - PROVIDE WASHERS BETWEEN ALL BOLT HEADS AND WOOD AND BETWEEN ALL NUTS AND WOOD.
 - MAXIMUM TRUSS SPACING = 2'-0" ON CENTER.
 - CENTROIDAL LINES OF MEMBERS SHALL INTERSECT AT A POINT UNLESS OTHERWISE NOTED.
 - ENDS OF ALL COMPRESSION MEMBERS SHALL BE CUT TO FIT INTERSECTING MEMBERS FOR POSITIVE END BEARING.
 - TRUSS JOINT CONNECTIONS SHALL BE MADE USING 20 GAGE, GALVANIZED CONNECTOR PLATES. THE TRUSS JOINT CONNECTOR PLATES SHALL BE SIZED BY THE FABRICATOR USING THE TRUSS MEMBER FORCES RESULTING FROM THE DESIGN LOADS SHOWN BELOW IN THE DESIGN CRITERIA.
 - THE CONTRACTOR SHALL PROVIDE ALL CONNECTIONS, CONNECTOR PLATES, ANCHORS AND ACCESSORIES AS REQUIRED FOR THE SUPPORT OF THE WOOD TRUSSES AND WOOD FRAMING.
 - WOOD TRUSSES SHALL BE CAMBERED AS REQUIRED FOR FULL DEAD LOAD DEFLECTION.
 - THE CONTRACTOR SHALL PROVIDE A DESIGN FOR REVIEW BY THE ARCHITECT AND/OR ENGINEER FOR THE WOOD TRUSSES FOR THE FULL TRUSS DEAD LOAD AND THE DESIGN LOADS GIVEN DESIGN LOADS SECTION.
 - TRUSS BRACING SHALL BE INSTALLED AS REQUIRED PER BCSI 1-03 AS PRODUCED BY WOOD TRUSS COUNCIL OF AMERICA AND TRUSS PLATED INSTITUTE.

FOUNDATIONS:

- FOUNDATION NOTES:
- ALL FOOTINGS SHALL BE POURED ON COMPACTED SOIL WITH A MINIMUM BEARING CAPACITY OF 1500 PSF.
 - CONTINUOUS WALL FOOTINGS SHALL BE POURED MONOLITHICALLY WITH COLUMN FOOTINGS.
 - ALL BACKFILL MATERIAL SHALL BE FREE OF DEBRIS. PLACE FILL IN 8" LIFTS WITH COMPACTION BETWEEN LIFTS TO A MINIMUM OF 95% OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT.
 - INSTALL FOUNDATIONS PER GEOTECHNICAL ENGINEER'S REPORT AND FIELD INSTRUCTIONS.
 - FOUNDATION TYPE AND DESIGN MAY BE FIELD-MODIFIED BASED UPON GEOTECHNICAL ENGINEER'S SITE DETERMINATIONS.
 - NOTIFY ENGINEER OF ALL FIELD DETERMINED CONCLUSIONS.
 - WALLS ACTING AS RETAINING WALLS SHALL NOT BE BACKFILLED WITHOUT BRACING UNTIL ALL SUPPORTING SOIL AND SLABS ARE IN PLACE.

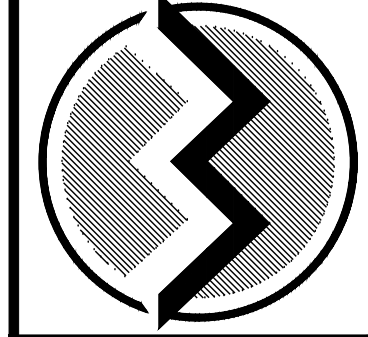
CONCRETE:

- CONCRETE NOTES:
- ALL CONCRETE IS TO BE PROPORTIONED AND PLACED IN ACCORDANCE WITH THE LATEST EDITION OF THE AMERICAN CONCRETE INSTITUTE/ACI 318.
 - CONCRETE SHALL HAVE MINIMUM COMPRESSIVE STRENGTH:
SLABS: 3000 PSI FOUNDATIONS: 3000 PSI FOOTINGS: 3000 PSI
 - ALL CONCRETE STEEL REINFORCEMENT TO BE GRADE 60.
 - ALL INTERIOR SLABS TO BE 4" THICK MINIMUM.
 - CONTROL JOINTS ARE TO BE PLACED AS SPECIFIED ON PLANS BOTH DIRECTIONS.

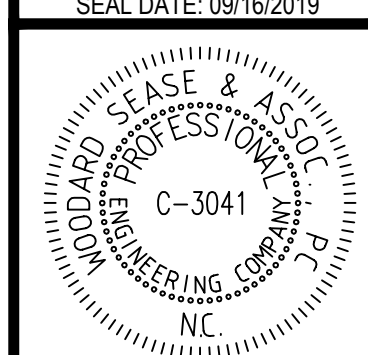
WOOD FRAMING NOTES:

- WOOD FRAMING NOTES
- ALL WOOD FRAMING SHALL BE KILN DRIED NO. 2 SPRUCE-PINE-FIR (OR BETTER) IN SIZES AS SHOWN ON THE PLANS, UNLESS OTHERWISE NOTED.
 - PLYWOOD AND ORIENTED STRAND BOARD (OSB) FOR SHEATHING SHALL COMPLY WITH USDOC PS 1 & PS 2, LATEST EDITION, RESPECTIVELY, AND SHALL HAVE AN APPROPRIATE GRADE TRADE MARK OF AN AMERICAN PLYWOOD ASSOCIATION/ENGINEERED WOOD ASSOCIATION APPROVED AGENCY ON EACH PANEL. PANELS SHALL BE APA RATED SHEATHING, EXPOSURE 1. SHEATHING SHALL BE APPLIED IN FULL SHEETS, OR IN LARGEST PIECES FOR THE AREA BEING COVERED.
 - ALL NAILING FOR WOOD CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE NORTH CAROLINA BUILDING CODE'S FASTENING SCHEDULE UNLESS OTHERWISE NOTED ON THE PLANS.
 - ALL PREFABRICATED JOIST HANGERS AND FRAMING ANCHORS SHALL BE FULLY NAILED.
 - ALL LAMINATED VENEER LUMBER (LVL) BEAMS SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES:
MODULUS OF ELASTICITY (E): 1,900,000 PSI
FLEXURAL STRESS (FB): 2600 PSI
COMPRESSION PERP. TO GRAIN (FC): 750 PSI
HORIZONTAL SHEAR (FV): 285 PSI

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SEAL DATE: 09/16/2019



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WOODARD SEASE & ASSOCIATES ASSUMES NO LIABILITY FOR DEVIATIONS FROM OR MODIFICATIONS MADE TO THE PLANS BY OTHERS. WOODARD SEASE & ASSOCIATES WILL NOT BE HELD RESPONSIBLE FOR CONTRACTORS FAILURE TO CONFORM TO CONSTRUCTION DOCUMENTS, FAILURE TO NOTIFY ENGINEERS OF KNOWN DISCREPANCIES, OR CONSTRUCTION MEANS AND METHODS.

JAMES A. CAMPBELL PARK
115 WEST JONES STREET, FUQUAY-VARINA, NC 27526
NEW TOILET BUILDING FOR THE TOWN OF FUQUAY-VARINA
ARCHITECT: BRYSON DESIGN, INC. PLAN: 2019-FVJC

NO.	DATE	BY	CHKD.
19-56-152	09/16/19		
DESIGNED	WPS	REV #	DATE
CHKD BY	WPS	1	-/-/-
CHKD BY	WPS	2	-/-/-
CHKD BY	BEW	4	-/-/-

SCALE: 1/4" = 1'-0"
DETAILS

SHEET:
2
2 OF 2